

VOLCAN LAND & WATER COMPANY

WARNER RESERVOIR CAPACITIES

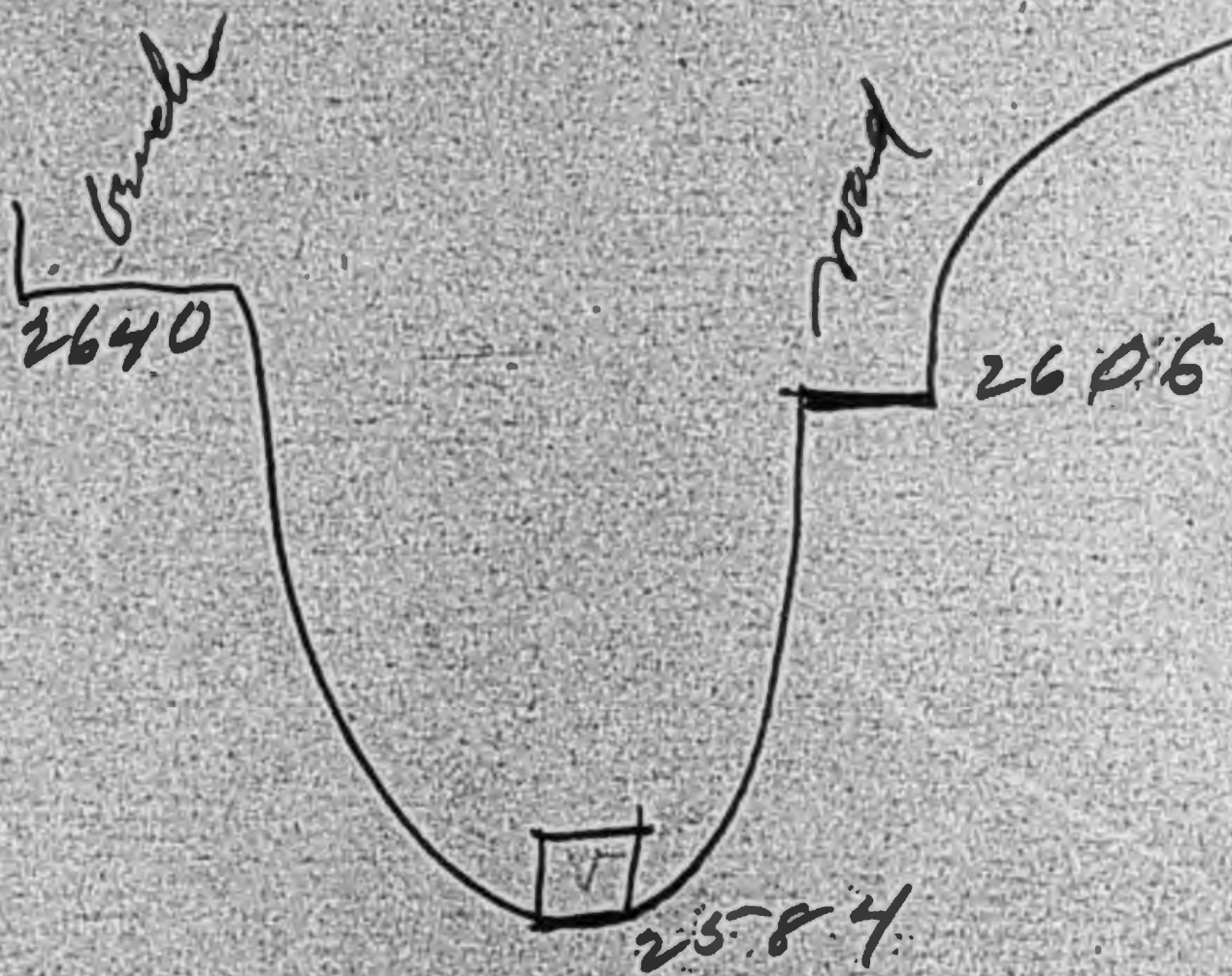
July 17, 1917

<u>Cont our</u> <u>U.S.G.S.</u>	<u>Depth</u>	<u>Acres</u> <u>Flooded</u>	<u>Total</u> <u>Acres Feet</u>
2620	0	0	0
2630	10	17	58
2640	20	58	453
2650	30	260	2,023
2660	40	875	7,698
2670	50	1,405	19,098
2680	60	1,822	35,233
2690	70	2,300	55,845
2700	80	2,960	82,145
2710	90	4,010	116,995
2715	95	4,560	138,420
2720	100	5,240	163,170
2727	107	6,080	203,140

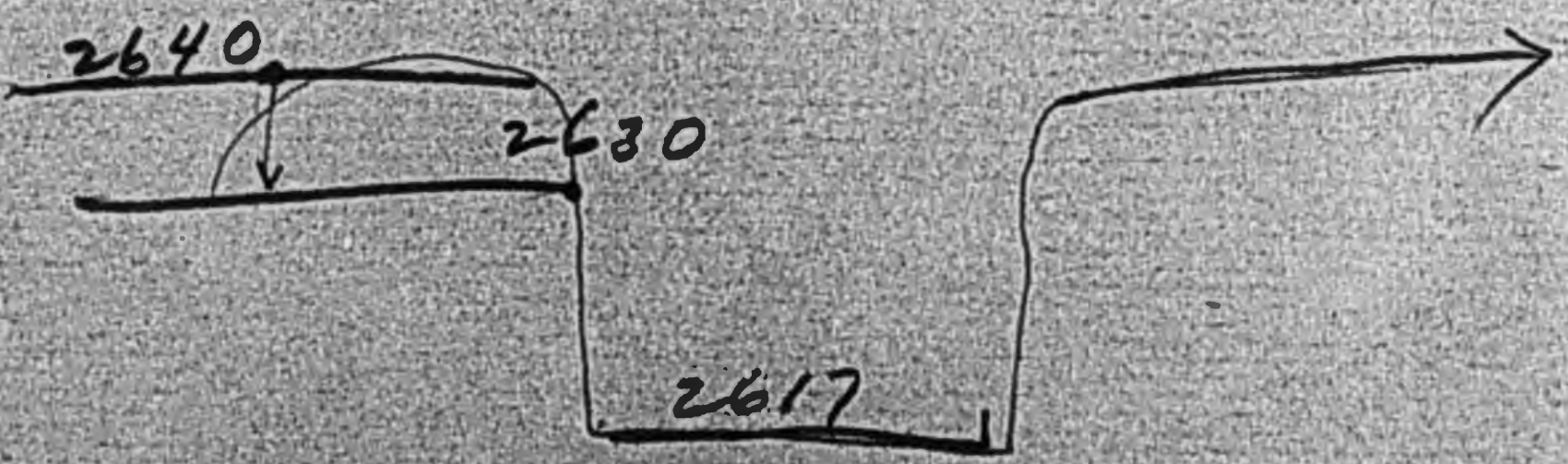
LLLLLLLLLLLL

26.	<u>2680</u>
	2644 ✓
	<u>278</u>
	<u>2560</u>

Sta 5300



House 2725
 BM 2724.63



Names Out off wall

LOGS OF TEST HOLES.

AT WARNER'S DAM, V. L. & W. CO.

BORINGS BY BYLLESBY & CO.

October-1911 - May-1912.

December 17.

Two days ago the drilling gang completed hole No. 5, (60' deep) and I asked for the reports as I had done before, at the completion of each hole. The reports for the first 41' were missing Mills took everything with him, Dec. 5th, when he left and reports for first of hole No. 5, went too. Larson says it was all disintegrated granite.

December 23.

Drillers are down 30' on hole No. 6.

COMPLETE RECORD. HOLE NO. 6. TEST BORINGS.

Located 25 ft. N. of hole No. 4. Started Dec. 16, finished Jan 23.

Depth ft.	Description
0 - 10'	Concrete
10 - 15'	Hard granite. Solid piece core 3' long.
15 - 18'	Hard broken granite.
18 - 27'	Hard broken granite.
27 - 37'	Very broken granite with seams of sand.
37 - 44'	Hard broken granite.
44 - 52'	Very broken granite. Hole caving badly fills up with sand. 12' had hard time getting tool out.
52 - 60'	Very broken granite.
60 - 62'	Very soft. Did not get any core.
62 - 64'	Hard broken granite. Put 3" casing to 62'. Proceeding with smaller drill.
64 - 68'	Hard broken granite.
68 - 78'	Very soft broken granite.
78 - 97'	Hard broken granite.
97 - 107'	Hard granite.
107 - 121'	Hard broken granite.
121 - 130'	Soft broken granite.
130 - 135'	Hard granite solid.
135 - 136'	Badly broken granite.
136 - 147'	Hard granite solid.
147 - 149'	Soft broken granite.

TEST BORINGS 2 WARNER'S CUT-OFF WALL.

Hole No. 7. 25' south of Hole No. 2. on C.L. of wall.

Depth ft.	Description
0 - 19'	Concrete.
19 - 24'	Disintegrated granite.
24 - 34'	Sand - no core.
34 - 35'	Hard broken granite.
35 - 41'	Hard & broken - very little core.
41 - 44'	Hard granite.
44 - 49'	Very soft with sand seams, no core.
49 - 50'	Very soft, no core.
50 - 52'	Hard solid granite.
52 - 54'	Very broken granite.
54 - 56'	Hard broken granite. hole caving badly.
56 - 60'	Hard broken granite. Hole fills with sand 8 or 10' when rod is taken out.
60 - 68'	Hard solid granite.
68 - 69'	Very broken granite.
69 - 81'	Disintegrated granite, did not get any core.
81 - 83'	Hard broken granite, hole caving bad, very little core.
83 - 87'	Very broken granite, very little core.
87 - 94'	Very broken granite, hole caving bad, fills 15'.
94 - 100'	Ground very broken and caving bad. Did not get any core. Had to use jar to get rod out. Hole fills in 20' with sand and hard rock over night.
100 - 103'	Took 5 hours getting to bottom of hole. Ground broken. Did not get any core.

End of Hole No. 7. February 12, 1912.

Drill hole No. 8. Feb 18 - Mar 9, 1912. 56' N. of Hole No 1.

Depths are measured down from top of highest concrete shoulder, Elevation 2626.0.

0 - 12'	Disintegrated granite.
12 - 14'	Granite boulder.
14 - 44'	Disintegrated granite.
44 - 59'	Very broken granite. Getting very little core.
59 - 62'	Very broken granite. Hole caving bad.
62 - 68'	Very broken granite with sand seams.
68 - 78'	Hard broken granite. Broken quartz with sand seams.
78 - 81'	Hard broken granite. Hole caving so bad could not get rods down. Cemented hole.
81 - 86'	Hard broken granite. Big pieces of granite caving in. Cement did not set.
86 - 98'	Hard broken granite.
98 - 101'	Very broken granite. Very little core.
101 - 108'	Hard solid granite.

End of Hole No. 8 Mar. 16, 1912.

Drill hole No. 9. Hole No. 9 - 82' S Hole No 5. Elev. of reference point for depths - 2663. Distance of hole from vertical face of shoulder at automatic gage - 57' horiz/

0 - 15'	Decomposed granite.
15 - 22'	" " mixed clay seams.
22 - 27'	" " and sand.
27 - 40'	Disintegrated granite with blue clay and sand.

Hole No 9. cont'd.

Depth ft.

40 - 45 Disintegrated granite mixed with clay.
45 - 60 " " and fine sand.
60 - 70 " " fine sand and blue clay.
70 - 74 Very broken hard granite.
74 - 79 Hard broken granite.
79 - 90 Disintegrated granite.
90 - 97 " " and blue clay.
97 - 101 Broken granite and sand seams.
101 - 108 Very broken granite.
108 - 113 Disintegrated granite and sand seams.
113 - 123 " " fine sand and blue clay. Very
small core.

Engine shaft broke. Now waiting for new shaft.
April 27, 1912.

Mesa Grande, Cal.

July 17-1911.

Mr. H. Hawgood,
Los Angeles Cal.

My dear Mr. Hawgood:

I enclose herewith excavation
progress sheet.

I have been obliged to go up
and tighten ^{above} up the water seepages
in the main Creek or Agua ^{Cabiente} ~~Fobia~~ & am
preparing to put in the flume
over to the south Branch or ^{Carrizo Creek} ~~Agua Fobia~~
as the pump cannot lower the
water plane beyond 5 ft. at present
without continuous pumping. As I
have been using the pump for sluicing
down all that can be moved of the
high north side, ~~it is~~ & much water
has returned to the sump, this does

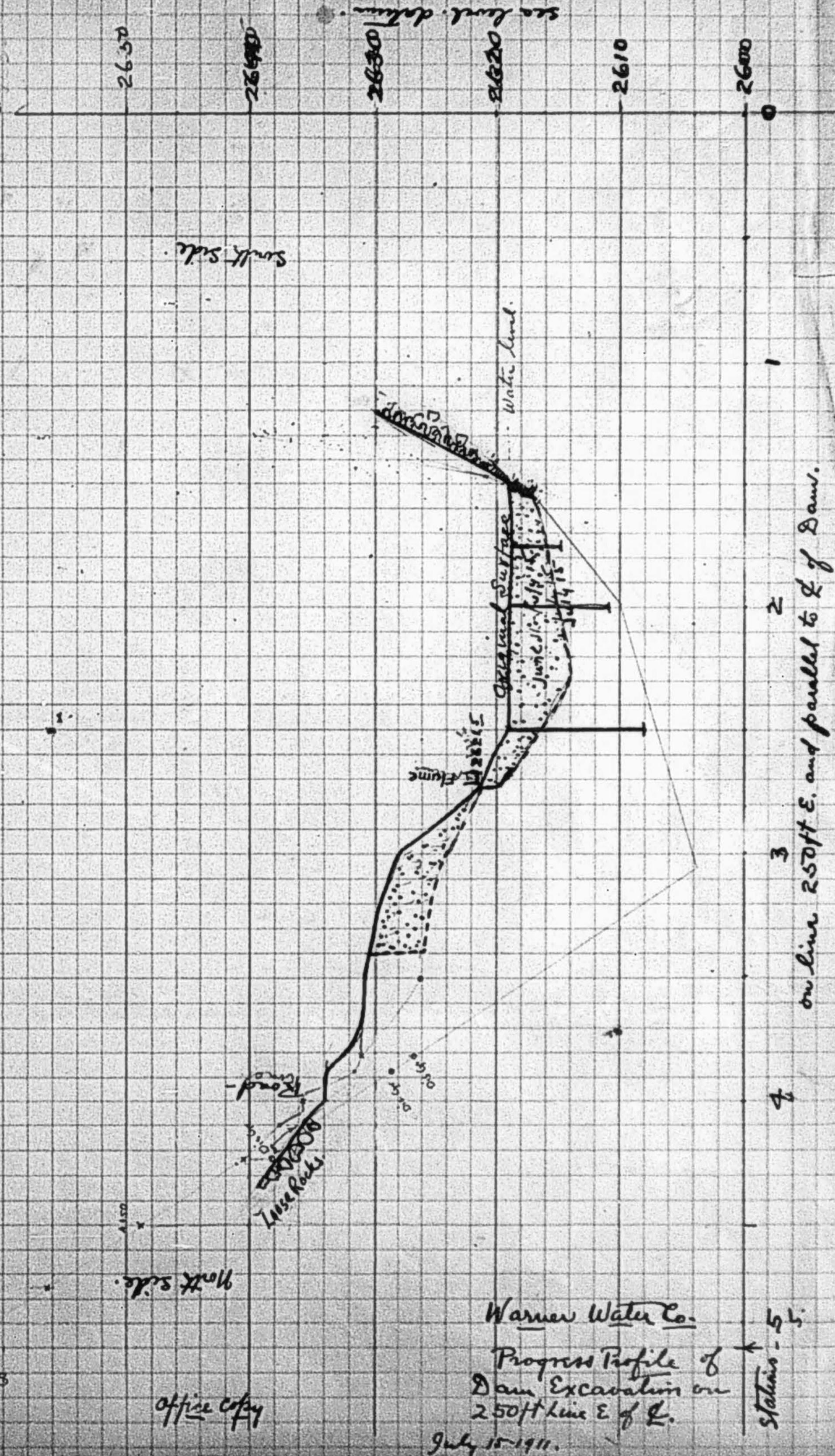
not mean that we have reached our pump capacity, but this combined with a 50% increase of water ^{due to rains} has obliged makes necessary ~~some~~ cutting off preliminary work. more water.

I think with these rearrangements ^{by the end of week} we should be down to the borings between stations 1+50 to 2+75.

We are getting a six inch layer of peat about 3 ft down, but the rest is uniform coarse sand.

Very Sincerely Yours

3.3
2.4
2.7
1.87



Copy

Mesa Grande, Cal
July 17-1911.

Mr. H. W. Keller
Los Angeles Cal.

Dear Sir:

I enclose a profile of progress
on cut to July 15th.

The trench has to be some
sixty feet wide at the top, gradually
narrowing of course as we make depth.
So the yardage to be moved near the
surface is considerable.

The water has increased from
100 minus inches surface flow to about
150 ~~to~~ ^{to} inches on account of summer
rains. I have decided to put in a
5ft dam in the narrows just below the
ford, to protect the work done against
heavy rain or cloud burst.

The rainfall ~~to~~ ^{to} July 17th is .87 inch,
at the Dam site.

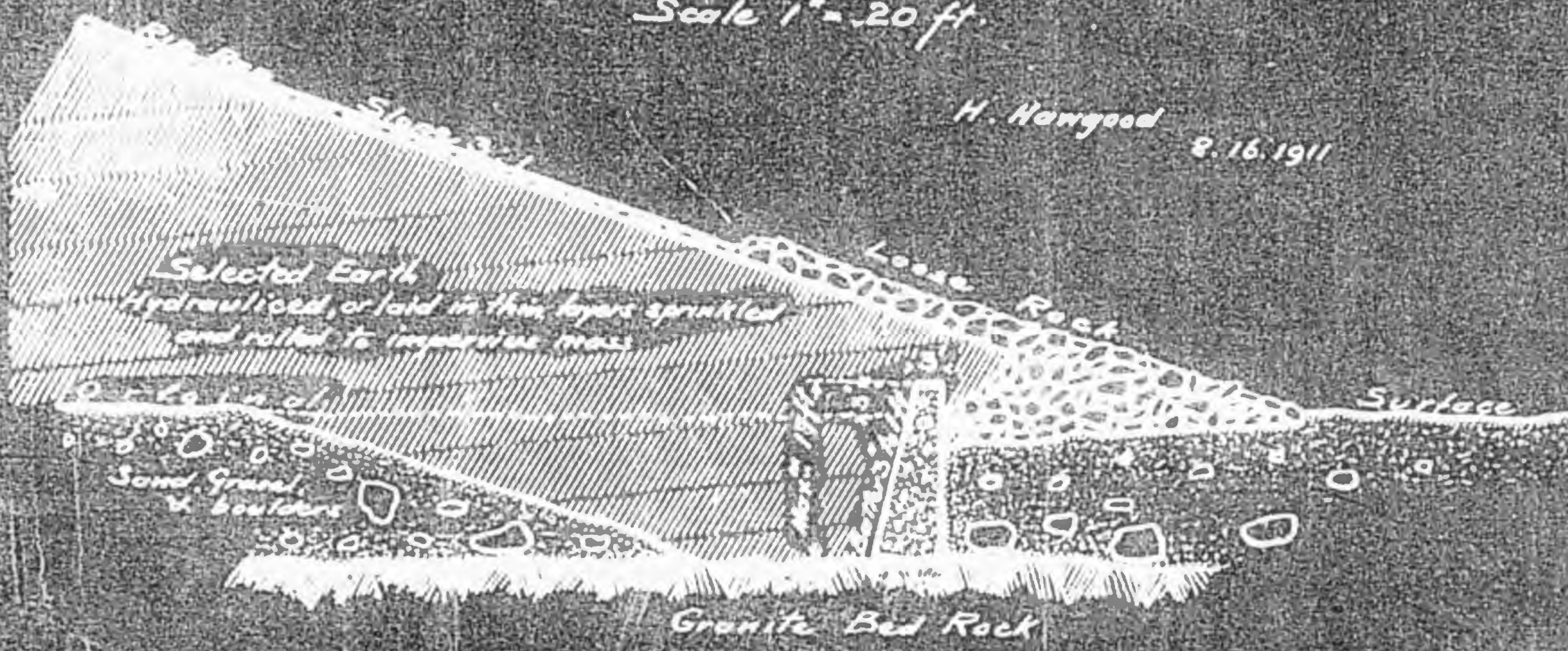
Very Sincerely yours
W. S. Post

Warner Dam

Concrete Cut-off Wall

Scale 1" = 20 ft.

H. Hawgood 8.16.1911



Warner Dam. Progress for week ending Oct 14			
Date	no. of Labour.	Character	Where - Concrete yds.
Oct. 9	14	Excavation.	South end.
" 10	14	Expn + Road.	"
" 11	14	Excavation	" 4
" 12	15	Timbering + Concr.	" 19
" 13	13	Concr + Excav.	" 28
" 14	9	" "	" 9

Total for week 51
 Total ~~blank~~ place last week. 295
 Total to date 346
 % completed 46% - about.

should have been $\frac{30}{2}$
~~353~~
 362 →

all deep excavation hoisted on Winze used for deep timbering continued - but pressure due to clays' severe + several caves, including dropping pump frame. 2 men on hand pumps in addition to 4 on engine. Work ends with water cut off and south end 25 ft deep below bed partial work. Ponds continuous work showing along upper toe of large piers. Decided to erect

and some further etc.

295
147
442

346
147
493

9 to add -

Progress week ending Oct 21, 1911

39
24
21
84
5
21
31
14

Date -
Oct 16
" 17
18
19
20
21

No of Laborers -
9
14
12
17
15
15

Character
Concrete
Concrete + Earth
Concrete
" rock
"
Concrete + Earth

When -
Blk 19
" 20
" 20+21
" 21+22
" 22+23

Concrete yds -
25 ✓ 25
29 ✓ 32
21 ✓ 21
31 - 25
35
29

Total Reported Oct 14 346
Omissions 7th + 14th 16
Corrected Total - 362

167
362
529

70% completed.

Cost of

WARNERS RANCH INVESTIGATION

by Prylleby & Co. - Results of Borings furnished
to V.L.W. Co. & are now in their possession. + cores were

Legal		26,492.18	
Report of M. M. O'Shaughnessy		1,122.85	
Traveling Expenses		5,635.71	
Engineering Services		9,301.85	
Automobile Hire		422.40	
Miscellaneous, including freight, telegraph, telephone, etc.		830.66	
Drilling Outfit		1,980.81	
Camp Outfit		139.15	
Drilling			
Labor	\$3,777.08		
Material, supplies and repairs	1,033.52		
Boarding	979.82		
Insurance	159.50		
Taxes	11.60		
		<u>5,961.52</u>	\$31,887.13
Less Credit:			
Drilling machinery returned			<u>1,152.62</u>
			\$30,734.51

COST OF WARNERS RANCH INVESTIGATION
BY BYLLESBY & COMPANY

Results of Borings and Cores were furnished
to Volcan Land & Water Company and are now
in their possession.

Legal		\$6,492.18
Report of M. M. O'Shaughnessy		1,122.85
Traveling Expenses		5,635.71
Engineering Services		9,301.85
Automobile Hire		422.40
Miscellaneous, including freight, telegraph, telephone, etc.		830.66
Drilling Outfit		1,980.81
Camp Outfit		139.15
Drilling		
Labor	\$5,777.08	
Material, supplies and repairs	1,033.52	
Boarding	979.82	
Insurance	159.50	
Taxes	11.60	
	<u>5,961.52</u>	\$51,887.15
Less Credit:		
Drilling machinery returned		<u>1,152.62</u>
		\$50,734.51

Post estimate

		Unit	Price	Quantities	Quantity	Am't.
VII. Pamo Dam.						
(a)	Hydraulic fill type					
1.	Material for Hyd. fill in place.....	cu yd	.15	958,000	958,000	\$ 143,700
2.	Concrete for outoff walls, slopes advised <i>slopes advised</i>	2 1/2 cu yd	\$ 7.7	3,000		21,000
(b)	Loose Rock fill type;					
1.	Loose rock	cu yds	.65			
2.	Concrete for outoff walls, etc.,.....	cu yds	\$ 7			
3.	Planking up stream face in place.	1,000 B. M.	\$ 50			
(c)	Masonry Type.					
1.	Concrete rubble	cu yds.	\$ 4	121,000		
(d)	Foundation excavations					
1.	Sand	cu. yds.	} .30	60,000	60,000	18,000
2.	Disintegrated Granite etc.,.....	lump sum				
(e)	Outlet Tower, Grates, etc.		\$ 50,000			
(f)	Alternative shaft		\$ 40,000			
	Gate Chamber Gates, etc.	lump sum				40,000
						\$ 222,700

includes 800 gravel

Gates Shaft. includes tunnel outlet - 800ft.

5.	Unit	Price	Quantity
VIII. Pamo Conduft.			
1 Paved Ditch 13 miles.....	13 miles	\$ 15,000	13
2. 36" Steel Pipe 5660 Lin ft.	5660 Lin ft.	\$ 3	5660.
3. 42" dia. wood pipe 9 1/2 miles	9 1/2 miles	\$ 16,000	9.5
4. 80 Air valves	Each	\$ 3	80
5. 25 Blow offs	"	\$ 15	25
6. Lumber for pipe treat- les, 10,000' B.M.	10000' B.M.	\$ 50	10 m.
7. Flume			

IX. Miscellaneous

1. Engineering & Supt.	5%		
2. Roads	miles	\$ 500	20
3. Rights of Way	—	—	—
4. Telephone.	miles	150	25

Post Estimate	Quantity	Amount
	13	\$ 215,000
	5660	17,000
	9.5	161,500
	80	300
	25	400
	10 m.	500.
		<u>\$ 394,700</u>

Alverson Final Estimate
 Quantity ^{made 5.28'}
 4' x 10' x 4'
 Redwood Flume.
 @ 2.5 cent/ft.

		20,000
		10,000
		—
		3,800
		<u>\$ 428,500</u>

Center Line of Warner Dam.

Sta.	Elev.	above or Ft. below top of dam.
0+00	2712.53	+2.53
0+25	2712.06	+2.06
0+50	2716.68	+6.68
0+75	2712.87	+2.87
0+83	2710.0	0.00
1+00	2701.99	-8.01
1+25	2686.56	-23.44
1+50	2671.76	-38.24
1+75	2649.26	-60.74
2+00	2624.29	-85.71
2+07	2618.66	-91.34
2+25	2617.56	-92.44
2+50	2617.76	-92.24
2+75	2618.16	-91.84
3+00	2618.06	-91.94
3+25	2620.02	-89.98
3+50	2626.54	-83.46
3+75	2625.63	-84.37
3+85	2626.20	-83.80
4+00	2633.20	-76.80
4+25	2653.68	-56.34
4+50	2671.16	-38.84
4+75	2678.22	-31.78
5+00	2682.77	-27.23
5+25	2688.64	-21.36
5+50	2695.47	-14.53
5+75	2705.02	-4.98
5+90.5	2710.00	0.00
6+02	2714.51	+4.51

bottom of hill

bottom of hill

Note
 Station 0+00 is at S. end of center line of top of dam.
 Bearing of center line is (N 10° 48' E) (true bearing)
 Top of dam is 20ft wide.
 3-1 slope up stream.
 2-1 slope down stream.

N 3 Line

N 87° 11' E
 80 11
 N 70° 00' E ?

Line 1+00			Line 1+75 cont'd.		
25 E	2700.0	10.0	275 E	2621.0	89.0
50 E	2699.1	10.9	25 W	2652.1	57.9
25 W	2701.4	8.6	50 W	2655.7	54.3
Line 1+25			75 W	2658.9	51.1
25 E	2684.4	25.6	100 W	2660.8	49.2
50 E	2675.5	34.5	125 W	2677.0	33.0
75 E	2672.8	37.2	Line 2+00		
100 E	2669.5	40.5	25 E	2620.3	89.7
125 E	2665.4	44.6	50 E	2617.8	92.2
150 E	2658.7	51.3	75 E	2618.1	91.9
175 E	2649.9	60.1	100 E	2618.5	91.5
25 W	2688.5	21.5	125 E	2619.1	90.9
50 W	2690.2	19.8	150 E	2618.8	91.2
75 W	2692.2	17.8	175 E	2619.2	90.8
Line 1+50			200 E	2619.6	90.4
25 E	2662.6	47.4	225 E	2620.1	89.9
50 E	2662.7	47.3	250 E	2620.7	89.3
75 E	2654.4	55.6	275 E	2621.0	89.0
100 E	2645.6	64.4	25 W	2628.0	82.0
125 E	2635.8	74.2	50 W	2632.6	77.4
150 E	2631.7	78.3	75 W	2637.4	72.6
175 E	2623.0	87.0	100 W	2637.3	72.7
200 E	2622.5	87.5	125 W	2640.3	69.7
225 E	2620.0	90.0	150 W	2645.1	64.9
250 E	2620.5	89.5	Line 2+25		
275 E	2621.0	89.0	25 E	2617.8	92.2
25 W	2672.8	37.2	Uniform grade to	89.0	
50 W	2673.9	36.1	275 E	2621.0	
75 W	2679.1	30.9	25 W	2617.5	92.5
100 W	2678.9	31.1	50 W	2617.4	92.6
Line 1+75			75 W	2621.5	88.5
25 E	2642.7	67.3	100 W	2624.2	85.8
50 E	2640.3	69.7	125 W	2626.3	83.7
75 E	2625.4	84.6	150 W	2625.7	84.3
100 E	2621.9	88.1	175 W		
125 E	2618.7	91.3			
150 E	2618.1	90.9			
175 E	2619.5	90.5			
200 E	2619.9	90.1			
225 E	2620.4	89.6			
250 E	2620.7	89.3			

Sheet 2.

Line 2+50			Line 3+50		
25 E 2617.9	92.1		25 E 2626.5	83.5	
Uniform grade to			50 E 2628.0	82.0	
275 E 2621.0	89.0		75 E 2629.2	80.8	
25 W 2617.6	92.4		100 E 2631.1	78.9	
Uniform grade to			125 E 2632.6	77.4	
200 W 2619.9	90.1		150 E 2631.7	78.3	
Line 2+75			175 E 2630.4	79.6	
275 E 2621.0	89.0		200 E 2630.8	79.2	
Uniform grade to			225 E 2631.3	78.7	
200 W 2616.6	93.4		250 E 2633.5	76.5	
Line 3+00			25 W 2626.3	83.7	
25 E 2618.4	91.6		50 W 2624.5	85.5	
50 E 2618.9	91.1		75 W 2621.7	88.3	
75 E 2619.7	90.3		100 W 2620.5	89.5	
100 E 2620.7	89.3		125 W 2619.6	90.4	
125 E 2621.2	88.8		150 W 2617.4	92.6	
150 E 2623.5	86.5		175 W 2617.3	92.7	
175 E 2627.6	82.4		200 W 2616.3	93.7	
200 E 2628.2	81.8		Line 3+75		
225 E 2630.7	79.3		25 E 2626.2	83.8	
250 E 2633.1	76.9		50 E 2627.3	82.7	
25 W 2617.8	92.2		75 E 2628.5	81.5	
Uniform grade to			100 E 2629.7	80.3	
200 W 2616.5	93.5		125 E 2631.4	78.6	
Line 3+25			150 E 2631.7	78.3	
25 E 2620.4	89.6		175 E 2631.1	78.9	
50 E 2620.6	89.4		200 E 2631.8	78.2	
75 E 2622.7	87.3		225 E 2631.9	78.1	
100 E 2624.5	85.5		250 E 2632.9	77.1	
125 E 2625.3	84.7		25 W 2625.1	84.9	
150 E 2626.1	83.9		50 W 2625.8	84.2	
175 E 2626.7	83.3		75 W 2626.9	83.1	
200 E 2628.4	81.6		100 W 2626.1	83.9	
225 E 2630.8	79.2		125 W 2625.3	84.7	
250 E 2633.4	76.6		150 W 2625.5	84.5	
25 W 2618.7	91.3		175 W 2620.8	89.1	
Uniform grade to			200 W 2621.2	88.8	
200 W 2616.4	93.6				

Sheet 3.

Line 4+00			Line 4+50 cont'd.		
25 E 2634.2	75.8		175 E 2641.3	68.7	
50 E 2634.9	75.1		200 E 2646.7	63.3	
75 E 2636.7	73.3		25 W 2673.8	36.2	
100 E 2636.9	73.1		50 W 2673.9	36.1	
125 E 2636.6	73.4		75 W 2666.3	43.7	
150 E 2637.0	73.0		100 W 2662.1	47.9	
175 E 2630.5	79.5		125 W 2656.5	53.5	
200 E 2635.6	74.4		Line 4+75		
225 E 2635.8	74.2		25 E 2674.4	35.6	
250 E 2635.9	74.1		50 E 2671.7	38.3	
25 W 2633.4	76.6		75 E 2671.9	38.1	
50 W 2631.4	78.6		100 E 2667.9	42.1	
75 W 2628.6	81.4		125 E 2663.1	46.9	
100 W 2626.6	83.4		150 E 2656.3	53.7	
125 W 2625.8	84.2		175 E 2646.5	63.5	
150 W 2625.0	85.0		200 E 2649.2	60.8	
175 W 2626.2	83.8		25 W 2679.9	30.1	
Line 4+25			50 W 2679.7	30.3	
25 E 2650.8	59.2		75 W 2677.2	32.8	
50 E 2650.4	59.6		Line 5+00		
75 E 2652.2	57.8		25 E 2681.3	28.7	
100 E 2650.5	59.5		50 E 2680.3	29.7	
125 E 2648.3	61.7		75 E 2677.1	32.9	
150 E 2643.4	66.6		100 E 2672.9	37.1	
175 E 2636.4	73.6		125 E 2665.9	44.1	
200 E 2641.0	69.0		150 E 2658.5	51.5	
225 E 2644.1	65.9		175 E 2652.2	57.8	
25 W 2656.5	53.5		200 E 2649.5	60.5	
50 W 2655.5	54.5		25 W 2684.6	25.4	
75 W 2650.5	59.5		50 W 2683.7	26.3	
100 W 2643.5	66.5		75 W 2681.5	28.5	
125 W 2640.6	69.4		Line 5+25		
150 W 2637.4	72.6		25 E 2688.2	21.8	
175 W 2632.2	77.8		50 E 2688.3	21.7	
Line 4+50			75 E 2686.6	23.4	
25 E 2666.5	43.5		100 E 2675.3	34.7	
50 E 2663.5	46.5		125 E 2665.7	44.3	
75 E 2662.3	47.7		150 E 2663.6	46.4	
100 E 2660.8	49.2		25 W 2689.4	20.6	
125 E 2656.8	53.2		50 W 2689.2	20.8	
150 E 2650.8	59.2		75 W 2687.0	23.0	

Sect.	Area. Sq.ft.	Vol. Cu.ft.	Bet. Sects.
0+83	0.0	2995.0	0+83-1+00
1+00	352.4	41000.0	1+00-1+25
1+25	2927.5	177940.0	1+25-1+50
1+50	11307.6	746485.0	1+50-2+00
1+75	14737.5	1079957.0	2+00-2+50
2+00	19320.5	1122705.0	2+50-3+00
2+25	21887.5	1037936.0	3+00-3+50
2+50	22724.3	900291.0	3+50-4+00
2+75	22704.9	344305.0	4+00-4+25
3+00	21180.7	223731.0	4+25-4+50
3+25	21000.6	233566.0	4+50-5+00
3+50	19369.2	62521.0	5+00-5+25
3+75	18134.7	32495.0	5+25-5+50
4+00	16126.9	14446.0	5+50-5+75
4+25	11417.5	2010.0	5+75-5+90.5
4+50	6481.0	Total = 6,022,383.0 Cu.ft. = 223,051.0 Cu.yds.	
4+75	4562.6		
5+00	3306.5		
5+25	1695.2		
5+50	904.4		
5+75	251.3		
5+90.5	0.0		

Line 5+50

25 E	2694.6	15.4
50 E	2693.8	16.2
75 E	2688.9	21.1
25 W	2697.2	12.8
50 W	2696.3	13.7

Line 5+75

25 E	2703.1	6.9
50 E	2697.5	12.5
25 W	2705.5	4.5

Line 5+96.4

10 E	2710.0	0.0
------	--------	-----

Line 5+87.5

10 W	2710.0	0.0
------	--------	-----

Cross-section for slope stakes

			5+96.4			0.0
			<u>0.0</u>			<u>10.0</u>
			5+90.5			
<u>0.0</u>			5+87.5			
<u>10.0</u>			<u>5.0</u>			<u>12.5</u>
<u>4.5</u>			5+75			<u>47.5</u>
<u>19.0</u>			<u>14.5</u>			<u>18.4</u>
<u>13.7</u>			5+50			<u>65.2</u>
<u>37.4</u>			<u>21.4</u>			<u>46.0</u>
<u>20.8</u>			5+25		23.4	<u>148.0</u>
<u>51.6</u>			<u>27.2</u>		<u>75.0</u>	<u>61.5</u>
<u>27.7</u>			5+00		57.8	<u>194.5</u>
<u>65.4</u>			<u>31.8</u>		<u>75.0</u>	<u>61.2</u>
<u>32.8</u>			4+75		42.1	<u>193.6</u>
<u>75.6</u>			<u>38.8</u>		<u>100.0</u>	<u>63.3</u>
<u>49.5</u>			4+50		49.2	<u>199.9</u>
<u>108.0</u>			<u>56.3</u>		<u>175.0</u>	<u>68.3</u>
<u>73.7</u>			4+25		57.8	<u>215.0</u>
<u>157.4</u>			<u>76.8</u>		<u>75.0</u>	<u>74.2</u>
<u>83.8</u>			4+00		73.3	<u>232.6</u>
<u>177.6</u>			<u>84.4</u>		<u>75.0</u>	<u>77.5</u>
<u>89.0</u>			3+75		77.9	<u>242.5</u>
<u>188.0</u>			<u>83.5</u>		<u>75.0</u>	<u>77.5</u>
<u>93.0</u>			3+50		82.0	<u>242.5</u>
<u>196.0</u>			<u>90.0</u>		<u>125.0</u>	<u>77.5</u>
<u>93.0</u>			3+25		83.3	<u>242.5</u>
<u>196.0</u>			<u>91.9</u>		<u>175.0</u>	<u>77.5</u>
<u>93.0</u>			3+00		84.6	<u>242.5</u>
<u>196.0</u>			<u>91.8</u>		<u>150.0</u>	<u>88.5</u>
<u>93.0</u>			2+75		91.1	<u>275.5</u>
<u>196.0</u>			<u>92.2</u>		<u>100.0</u>	<u>88.5</u>
<u>91.0</u>			2+50		92.0	<u>275.5</u>
<u>192.0</u>			<u>92.4</u>		<u>100.0</u>	<u>88.5</u>
<u>83.5</u>			2+25		89.8	<u>275.5</u>
<u>177.0</u>			<u>85.7</u>		<u>125.0</u>	<u>88.5</u>
<u>66.6</u>			2+00		92.2	<u>275.5</u>
<u>143.2</u>			<u>60.7</u>		<u>50.0</u>	<u>88.5</u>
<u>48.6</u>			1+75		91.3	<u>275.5</u>
<u>107.2</u>			<u>38.2</u>		<u>125.0</u>	<u>88.5</u>
<u>31.7</u>			1+50		87.0	<u>275.5</u>
<u>73.4</u>			<u>23.4</u>		<u>75.0</u>	<u>60.5</u>
<u>19.7</u>			1+25		25.2	<u>191.5</u>
<u>49.4</u>			<u>7.9</u>		<u>25.0</u>	<u>29.1</u>
<u>9.3</u>			1+00			<u>37.3</u>
<u>28.6</u>			<u>0.0</u>			<u>0.0</u>
			0+83			<u>10.0</u>
<u>0.0</u>			0+79			
<u>10.0</u>						

W. D. LOVELL
 1415 8TH STREET SOUTHEAST
 MINNEAPOLIS, MINNESOTA
 PHONES: N.W. EAST 692
 T.S. SPRUCE 241

Minneapolis, Minn.,
 Nov. 29, 1912.

Mr. W. S. Post,
 San Diego, Cal.

Dear Sir:

Referring again to the matter of foundations and building a dam at Warner's Dam site. Perhaps you will remember Byllesby's criticism of the dam foundations. It is a matter we have got to give careful consideration. In addition to the foundations for the dam there is the other difficulty, namely--the probability that water will escape through the disintegrated granite around the dam at the edges of the reservoir.

We ought to have some information concerning this possibility and probability. That water does percolate through disintegrated granite, there is no doubt, and that for a time at least after the dam is completed we would probably lose considerable water from seepage. Just how much no one can tell. If we can collect some data as to the rate of seepage through disintegrated granite, the data will be favorable to our project. So long as the matter rests without data it becomes a matter of opinion and any man can set up his opinion and although you know that his opinion is not worth anything, nevertheless it has its effect.

In the El Cajon Valley the residents are pumping ^{to} water out of open wells from 50 to 75 feet deep. The flow of these wells is through disintegrated granite. I happen to know that the flow is slow, but nevertheless it flows and is reasonably constant. The flow in disintegrated granite in the El Cajon Valley is probably not under much pressure. If this water was under say 80 feet of head, the flow would probably be much greater. We would also want to know if a greater flow would wash out the finer particles and disintegrate the balance so that in the end our dikes or wings would fail.

I have spent a lot of time on a scheme to make such tests as will determine these problems and answer them favorably to our cause. I have not yet satisfied myself just how to go about it. My personal opinion is that the reservoir itself will soon "silt up" so that there will be but little leakage, but my opinion simply is an opinion. In this connection you must remember what I said in a former letter about the character of that granite. Do you know of any water being stored in a reservoir which is built on this granite foundation? If so, what is the history?

Yours truly,



WDL:AAJ

V.L.W.Co. Engr.

Mesa Grande Cal.
Dec 10 - 1912

Mr. W. S. Post
San Diego - Calif.

Dear sir:

I am sending a box of rock samples, by express C.O.D. to you at #514 Am. Nat. Bank. I have labeled the samples in the box. I found several good samples which were excavated from the foundation about half way between where the ~~concrete~~ ^{scarp} was ~~stood~~ and the north end of the wall, about station 3+50 as I remember it. I also got a sample out of the test pit you had dug just above the road at the north end. I took some pieces of rock from the bank at the south end ^{of the wall} both at the water surface and some twenty feet in elevation above. The formation is crumbling ^{at the surface} and the samples are probably a little harder than the average. I tried to get as near the typical as I could.

Sincerely
H. L. Davis

California State Mining Bureau

FERRY BUILDING, SAN FRANCISCO, CAL.

F. McN. HAMILTON
W. H. STORMS STATE MINERALOGIST.

DEPARTMENT FOR DETERMINATION OF MINERALS.

NO 15009

Date, April 14th 1913

Name William S. Post P. O. San Diego
514 American Nat. Bank County " "
Specimen from Cutoff Wall, Bldg. Excavation about Station 3x50:
Wamer Dam

Rock badly decayed - minerals - Plagioclase is the chief primary mineral. Hornblende is fresh, Biotite plentiful, badly weathered. Quartz small amount chlorite secondary. Apatite chief accessory. The rock is a decomposed granodiorite.

(Over)

E. B. Preston
Determinative Mineralogist.

California State Mining Bureau

FERRY BUILDING, SAN FRANCISCO, CAL.

F. McN. HAMILTON
W. H. STORMS STATE MINERALOGIST.

DEPARTMENT FOR DETERMINATION OF MINERALS.

FILE

NO 15010

Date, April 14th 1913

Name William S. Post P. O.
County
Specimen from South end of cut off wall at present river bed
elevation 2620.

Rock has granitoid texture and is a granodiorite, carries plagioclase, between an oligoclase and andesine, Hornblende, Biotite, Quartz chlorite. Chief accessories are titanite and apatite, with secondary epidote and a zeolite as a seam filling - It is a plutonic rock.

(Over)

E. B. Preston
Determinative Mineralogist.

California State Mining Bureau

FERRY BUILDING, SAN FRANCISCO, CAL.

F. McN. HAMILTON
W. H. STORMS STATE MINERALOGIST.

DEPARTMENT FOR DETERMINATION OF MINERALS.

NO 15011

Date, April 14th 1913

Name William S. Post P. O.
County
Specimen from 15 feet below river bed.

Carries badly decomposed plagioclase, epidote (secondary), chlorite and quartz (Primary), with titanite, apatite as accessories. Rock badly altered, but not metamorphosed. Is a granodiorite.

(Over)

E. B. Preston
Determinative Mineralogist.

DEPARTMENT FOR DETERMINATION OF MINERALS

DETERMINATION OF MINERAL SAMPLES.

Samples (limited to three at one time) of any mineral found in the State may be sent to the Bureau for identification, and the same will be classified free of charge. No samples will be determined if received from points outside the State. It must be understood, that No Assays, or Quantitative Determinations will be Made. Samples should be in a lump form if possible, and marked plainly with name of sender outside of package, postoffice address, etc. No samples will be received unless charges are prepaid. A letter should accompany sample and a stamp should be enclosed for reply.

DEPARTMENT FOR DETERMINATION OF MINERALS

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COPY

CALIFORNIA STATE MINING BUREAU.

Ferry Building, San Francisco, Cal.

F. McN. Hamilton, State Mineralogist.

Department for Determination of Minerals.

No 15009. Date April 14th 1913.
Name William S. Post. P O San Diego
514 American Nat.
Bank Bldg. County " "

Specimen from Cutoff Wall, Excavation about Station 3x50
Warner Dam.

Rock badly decayed - Minerals - Plagioclase is the chief primary mineral. Hornblende is fresh, Biotite plentiful, badly weathered. Wuartz small amount chlorite secondary. Apatite chief accessory. The rock is a decomposed granodiorite.

E. B. Preston
Determinative Mineralogist.

COPY

CALIFORNIA STATE MINING BUREAU.

Ferry Building, San Francisco, Cal.

F. McH. Hamilton, State Mineralogist.

Department for Determination of Minerals.

No 15010.

Date April 14th, 1913.

Name William S. Post

Specimen from South End of cut off wall at present river bed,
Elevation 2620.

Rock has granitoid texture and is a granodiorite, carries
plagioclase, between an oligoclase and andesine, Hornblende,
Biotite, Quartz Chlorite. Chief accessories are titanite and
apatite, with secondary epidote and a zeolite as a seam fill-
ing - It is a plutonic rock.

E. B. Preston,

Determinative Mineralogist.

ESTIMATED COST OF WARNERS DAM

Height 95 feet; Top Elevation 2715 feet; Depth of water 90 feet.

October 8, 1914.

*Computed by Hickok.
O.K. W.S.P. May 1915*

- 000 -

Excavation,		
10350 cu.yds. at 70¢	-----	\$ 7,245.00
51800 cu.yds. at 25¢	-----	12,950.00
		<u>\$ 20,195.00</u>
Pumping	-----	5,000.00
Embankment - 336569 cu. yds at 30¢	-----	100,971.00
Facing of Concrete Slabs - 20700 sq.yds. at \$1.50	-----	31,050.00
Cut-off Wall	-----	30,000.00
Concrete Gutter - 600 lin. ft. at \$1.00	-----	600.00
6" Drain Tile - 2700 lin. ft. at 15¢	-----	405.00
Outlet Tunnel - 1020 lin. ft. at \$18.00	-----	18,360.00
Outlet Basin, Wier, etc.,	-----	3,000.00
Outlet Tower	-----	15,000.00
Footbridge to Tower	-----	1,000.00
Spillway on south side - 200' wide		
482 cu.yds.concrete at \$8.00	----	\$ 3,856.00
225 cu.yds.excavation at 50¢	----	112.00
Flashboard Arrangements	-----	500.00
		<u>4,468.00</u>
Spillway on north side - 100' wide		
48330 cu.yds.excavation at 30¢	--	14,500.00
3" Concrete Lining - 5900 sq. yds. at \$1.00	-----	5,900.00
Flashboards and foundations	-----	1,000.00
		<u>21,400.00</u>
Buildings	-----	6,000.00
Water Supply to Buildings	-----	1,000.00
Equipment, Tools	-----	4,000.00
Improvement of Grounds, Fencing, etc.,	-----	2,000.00
		<u>Total cost without overhead ----- \$264,449.00</u>

Deduct Expenditures to September 1st, 1914:

Cut-off Wall	-----	\$30,000.00
Outlet Tunnel	-----	9,050.00
Buildings	-----	6,000.00
Water Supply to Buildings	-----	1,000.00
Equipment, Tools	-----	4,000.00
Improvement of Grounds	-----	2,000.00
		<u>52,050.00</u>

\$212,399.00

Add 25% for engineering and contingencies ----- 53,100.00

\$265,499.00

Interest during construction:

8% for 4 months ($\frac{1}{3}$ period) ----- 7,080.00

Total cost to complete ----- \$272,579.00

Schedule of Work accomplished

- 1911.
- Jan 17 - Began survey to determine practicalness of the Lusardi - Temescal Divide. } 2 miles
- " 20± In Temescal Canyon. with -
Transmission, Cook, - + 4 men - pack
Camp -
- " 31 - Cost - in month - all survey
Exp Recd 60.30
Pay Roll 222.85
- Feb. 11 - Return to S.D.
- Mar - Temescal - Lusardi Survey } 6 miles
+ Pipe Line Power
- Apr. - Survey office - + consult with } 8 miles
Jeswhite - Hewins -
- May - start - Prospecting Warm Dam
" 20 - ~~stopped~~ temporarily on a high
in purchase.
- June 18 - Gas engine on ground & ready
June 21 -
Water going through by pass full on June 24.
Test borings June 26th

Mean Gross Evaporation Runoff & Accumulated Runoff
 Warners Reservoir - 2.8ft = 33.5 inches.

Drawing 434 [2]
 File No. K 60
 15c ft = 723.92 Acft pr. Yr.

July to July (Year)	Total 12 Station Rainfall	Evaporate Warners Reservoir	Actual Evaporation	in Feet Multiplier	Run off	Accumulated Run off
1894-1895	27.53	33.5	5.97	.50	71000	149310
1895-1896	11.15	33.5	22.35	1.86	4160	153470
1896-1897	21.64	33.5	11.86	.98	18750	172220
1897-1898	12.07	33.5	21.43	1.78	2080	174300
1898-1899	10.68	33.5	22.82	1.90	1100	175400
1899-1900	12.68	33.5	20.82	1.74	2080	177480
1900-1901	19.82	33.5	13.68	1.14	10400	187880
1901-1902	14.31	33.5	19.19	1.60	4160	192040
1902-1903	21.18	33.5	12.32	1.03	14560	206600
1903-1904	10.05	33.5	23.45	1.96	1872	208472
1904-1905	31.35	33.5	2.15	.18	53000	261472
1905-1906	32.08	33.5	1.42	.12	66960	328432
1906-1907	23.34	33.5	10.16	.85	52000	380432
1907-1908	19.31	33.5	14.19	1.18	7290	387722
1908-1909	22.02	33.5	11.48	.96	39500	427222
1909-1910	20.28	33.5	13.22	1.10	20800	448022
1910-1911	19.45	33.5	14.05	1.17	12500	460522
1911-1912	18.86	33.5	14.64	1.22	12129	472651
1912-1913	13.62	33.5	19.88	1.65	6042	478693
1913-1914	21.50	33.5	12.00	1.00	22200	500893
	382.92	670.0	287.08	23.92	500893	

Mean Gross Evaporation for Warners Reservoir = 2.8ft = 33.5 inches.
 Subtract from this the total Rainfall of 12 Station San Diego County Chart for each year - Difference will be net or actual Evaporation.
 This means that the Multiplier To get Evaporation Acre feet in the tabulation, will vary somewhat from year to year.

I consider the rainfall of the 12 Station chart as practically equal in Amount to what would fall on the Reservoir so it happens that it can be used without correction

July 10th 1914

W. S. Post. Eng.

Details for Mass Curve Warners Reservoir

[1]

Drawing 134
File No. F. 60

Based on being full at the beginning - 78310 Acft. =
assuming 17000 Acft. draft per year - 2349 Sq. ft.

		Acres flooded	Acft. flooded Total in Res.	Evaporation Multiplier	Total Evaporation	in Reservoir at end of Year	Accumulated Evaporation	Elevation of Water in Reservoir
Full	78310							
Run off	71000							
July to July								
1894-1895	149310	5370	10170					2719
Draft	17000							
	132310	4800	5085	.50	2542	129768	2542	2714
	2542							
1895-1896	129768							
Run off	4160							
in Res.	133928	4900	9130					
Draft	17000							
	116928	4230	4565	1.86	8491	108437	11033	2710
Evaporation	8491							
1896-1897	108437							
Run off	18750							
	127187	4620	8560					
Draft	17000							
	110187	3940	4280	.98	4194	105993	15227	2709
Evaporation	4194							
1897-1898	105993							
Run off	2080							
	108073	3880	7180					
Draft	17000							
	91073	3300	3590	1.78	6390	84683	21617	2703
Evaporation	6390							
1898-1899	84683							
Run off	1100							
	85783	3200	5900					
Draft	17000							
	68783	2700	2950	1.90	5605	63178	27222	2695
Evaporation	5605							
1899-1900	63178							
Run off	2080							
	65258	2500	4720					
Draft	17000							
	48258	2220	2360	1.74	4136	44122	31358	2687
	4136							
1900-1901	44122							
Run off	10400							
	54522	2350	4350					
Draft	17000							
	37522	2000	2175	1.14	2479	35043	33837	2683
	2479							
	35043							

July 9th 1914
 H. B. Smith

Drawing 246 [2]

Details for Mass Curve "Warners Reservoir" Continued File No. 60

Year	Acres Flooded	Reflooded Total Mean	Evaporation Multiplier	Total Evaporation	In Reservoir at end of Year	Accumulated Evaporation	Elevation of Water in Res.	
1901-1902	brought forward					35043	33837	2683
1901-1902	35043							
Run off	4160							
	39283	2070		3640				
Draft	17000					2820		
	22203	1570	1.60	2820		19383	36657	
Evaporation	2820						2673	
1902-1903	19383							
Run off	14560							
	33943	1920		3270				
Draft	17000					1684		
	16943	1350	1.03	1684		15259	38341	
Evap ²	1684						2667	
1903-1904	15259							
Run off	1872							
	17131	1380		1580				
Draft	17000					131		
	131	200	1.96	1545		38472	2644	
Evap ²	1545							
1904-1905	38472							
Run off	53000							
	53000	2330		1970				
Draft	37000					177		
	36000	1970	.18	1.77		35823	38649	
Evap ²	177						2683	
1905-06	35823							
Run off	66960							
	102783	3750		7030				
Draft	17000					422		
	85783	3280	.12	422		85361	39071	
Evap	422						2704	
1906-07	85361							
Run off	52000							
	137361	5100		9500				
Draft	17000					4038		
	120361	4400	.85	4038		116323	43109	
Evap ²	4038						2711	
	116323							

Plotted July 10th 1914

[Signature]

Drawing A34
File No F. 60
Year

Details for Mass Curie Warners Reservoir Continued

13

Acres Flooded Acres flooded Evaporation Evaporation in Reservoir Accumulated Elevation of
Total & Area Multiplier As feet at end of Year Evaporation Water in Res.

1906-1907	brought forward						116323	431.09	2711
1907-1908	116323								
Run off	7290								
	123613	4550	} 8450						
Draft	17000								4985
Evap ⁿ	106613	3900	4225	1.18	4985	101628	48094	2709	
	4985								
1908-1909	101628								
Run off	39500								
	141128	5220	} 9770						
Draft	17000								4690
Evap ⁿ	124128	4550	4885	.96	4690	119438	52784	2712	
	4690								
1909-1910	119438								
Run off	20800								
	140238	5180	} 9700						
Draft	17000								5335
Evap ⁿ	123238	4520	4850	1.10	5335	117903	58119	2711	
	5335								
1910-1911	117903								
Run off	12500								
	130403	4950	} 9100						
Draft	17000								5324
Evap ⁿ	113403	4150	4550	1.17	5324	108079	63443	2710	
	5324								
1911-1912	108079								
Run off	12129								
	120208	4400	} 8150						
Draft	17000								4972
Evap ⁿ	103208	3750	4075	1.22	4972	98236	68415	2707	
	4972								
1912-1913	98236								
Run off	6042								
	104278	3800	} 7050						
Draft	17000								5816
Evap ⁿ	87278	3250	3525	1.65	5816	81462	73231	2702	
	5816								
1913-1914	81462								
Run off	22200								
	103662	5760	} 7000						
Draft	17000								3500
Evap ⁿ	86662	3240	3500	1.00	3500	83162	76731	2703	
	3500								
	83162								

July 1914
K. H. Park

H. HAWGOOD
CONSULTING ENGINEER
H.W.HELLMAN BUILDING

M. AM. SOC. C. E.
M. INST. C. E. (LONDON)
M. AM. BY. ENG. & M. W. ASSOC.

LOS ANGELES, CALIFORNIA

June 30th, 1911.

Mr. W. S. Post,

Witch Creek, San Diego Co., Cal.

Dear Mr. Post:-

The first of our measurements at the lower dam site worked out 88.5 Miners Inches, and the second 91.0 Miners Inches, which is a very close agreement, and we can accept 90 inches as a reliable figure.

We measured in the flume 35 M. I., and assuming there was an equal amount of surface flow not yet taken in, we would have 70 inches accounted for, leaving 20 for underflow. I look forward with interest to what your float measurements gave.

I find that my old notes, dated 18 May, 1905, give "Granite ledges above dam site strike S. 34° E., dip 72° S. 56° W." Also, at west end of Monkey Hill, "Strike S. 50° E., dip nearly 90° Wly." The above courses are magnetic. The strike at the dam, as noted above, agrees with the direction of the ledge that you have discovered.

Yours very truly,

H. Hawgood

BUILDING WARNER DAM

From the papers of Ed Fletcher, the following letters were removed to the alphabetized correspondence files

BENT, Arthur S. to Fletcher, 9/21/22

KING, T.H. to Fletcher, 4/7/22

Fletcher to KLAUBER, Hugo, 4/7/22

STARR, R.E.

Fletcher to Starr, [4 letters] 4/4/22, 4/7/22, 4/18/22,
4/26/22

Starr to Fletcher, 4/15/22, with an attached letter, Starr
to Treanor, 4/15/20

SWALLOW, Charles H., to Fletcher, 9/2/22

Fletcher to SWYCAFFER, J.D., 4/14/22

THOMAS, Carl C., to Fletcher, 4/1/22

Fletcher to TREANOR, 4/15/22

WHITE, F.M. to Fletcher, 11/10/22

Ed Fletcher Papers

1870-1955

MSS.81

Box: 50 Folder: 11

**Business Records - Water Companies - Volcan
Land and Water Company - San Dieguito
System - Warner Dam (Lake Henshaw) and
associated projects - Construction Notes and costs**



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